



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Cover

Bob's Tile Depot

Design Loads

Description: New construction of retail store

Project Number: 26-1234

Structural Design Loads per: IBC 2024

Project Location

132 Granite Blvd
Sample Town, NM

Engineering Team

Engineer: ACB

Engineer of Record: WRB

Notes

New construction retail store with tilt-up walls, bar-joist roof



Project Title:	Bob's Tile Depot
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Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building Overview

Main Building

Design Loads

Risk Category: II

Structure Location

132 Granite Blvd
Sample Town, NM

Engineering Team

Engineer: ACB

Engineer of Record: WRB



Project Title:	Bob's Tile Depot
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Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Dead Load

Dead Load Assemblies

Typical Roof Load

Material	Weight
Waterproofing membranes - Bituminous, gravel-covered	5.50 psf
Insulation, Roof Boards - Polystyrene foam - 6 in.	1.20 psf
Deck, metal, 18 gauge	3.00 psf
20K Joists at 4ft OC	2.50 psf
Assembly Weight	12.20 psf

Tilt-Up walls

Material	Weight
7 1/4" Tilt-up Panel	87.60 psf
Assembly Weight	87.60 psf



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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Live Load

Live Loads Used

Stairs And Exit Ways

Use	Uniform	LL Reduction Permitted	Multistory LL Reduction Permitted	Concentrated
One And Two-Family Dwellings Only	40.0 psf	Yes	Yes	300.0 lbs

Roofs - General

Use	Uniform	LL Reduction Permitted	Multistory LL Reduction Permitted	Concentrated
Ordinary Flat, Pitched, And Curved Roofs	20.0 psf	Yes	N/A	0.0 lbs

Storage Warehouses

Use	Uniform	LL Reduction Permitted	Multistory LL Reduction Permitted	Concentrated
Light	125.0 psf	No	Yes	0.0 lbs



Project Title:	Bob's Tile Depot
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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Rain Load

Rain Loads

General Rain - Parameters

15 min. Rainfall intensity 5.48 in/hr ASCE 7-22: §8.2

Roof Region - Main Region

Main Region - Drainage System

Drainage System	Overflow Dam or Standpipe	<i>User Defined</i>
Inlet Diameter	6	<i>User Defined</i>
Outlet Diameter	4	<i>User Defined</i>

Main Region - Design Loads

$$R = 5.2(d_s + d_h + d_p) \quad \text{ASCE 7-22, (8.2-1)}$$

$$Q = 0.0104 A i \quad \text{ASCE 7-22, (C8.2-1)}$$

Drainage Collection Area (A)	Static Head Pressure (d_s)	Flow Rate (Q)	Hydraulic Head Pressure (d_h)	Ponding Head (d_p)	Design Rain Load (R)
2500.00sqft	1.50in	142.48gal/min	2.35in	2.25in	31.72psf
1325.00sqft	1.50in	75.51gal/min	1.26in	2.25in	26.03psf



Project Title:	Bob's Tile Depot
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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Snow Load

Snow Load Report

General Snow - Parameters

Ground Snow Load	$p_g = 20.0$ psf	ASCE 7-22: Figure s 7.2-1A through 7.2-1D
Building Risk Category	Risk Category = II	User Defined
Winter Wind Parameter	$W_2 = 0.5$	ASCE 7-22: Figure 7.6-1

Roof Region - Main Building

Main Building Design Summary

Balanced Flat Roof Snow Load	$p_f = 14.74$ psf	ASCE 7-22: §7.3
Balanced Sloped Roof Snow Load	$p_s = 14.74$ psf	ASCE 7-22: §7.4
Minimum Roof Snow Load	$p_m = 20.0$ psf	ASCE 7-22: §7.3.3
Rain on Snow p_s Surcharge	$p_{s\text{surcharge}} = 8.0$ psf	ASCE 7-22: §7.10
Drift Loads for Roof Steps		ASCE 7-22: §7.7

Location	Length of roof upwind of the drift (lu)	Length of roof downwind (ld, lower)	Height of roof step (h)	Uniform pressure, windward face (pw)	Uniform pressure, leeward face (pL)	Height of drift (hd)	Pressure surcharge on leeward face (pLS)	Leeward Surcharge width (ds)
E-W East Roof Step	350.0 ft	400.0 ft	6.25 ft	62.72 psf	79.81 psf	3.78 ft	79.81 psf	30.23 ft

Drift Loads for Roof Projection Parapets ASCE 7-22: §7.8

Location	Length of roof upwind of the drift (lu)	Length of roof downwind (ld, lower)	Height of roof step (h)	Uniform pressure, windward face (pw)	Uniform pressure, leeward face (pL)	Height of drift (hd)	Pressure surcharge on leeward face (pLS)	Leeward Surcharge width (ds)
E-W West parapet	1.0 ft	400.0 ft	2.25 ft	22.61 psf	7.7 psf	1.36 ft	22.61 psf	10.9 ft
N-S High Parapet	1.0 ft	300.0 ft	5.75 ft	56.72 psf	7.7 psf	3.42 ft	56.72 psf	27.33 ft
N-S Low Parapet	1.0 ft	300.0 ft	2.25 ft	22.61 psf	7.7 psf	1.36 ft	22.61 psf	10.9 ft

Main Building Design Detail

Balanced Flat Roof - ASCE 7-22: §7.3

$$p_f = 0.7C_eC_t p_g$$

ASCE 7-22: Eq. 7.3-1

Exposure Factor

$$C_e = 0.9$$

ASCE 7-22: §7.3.1



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Subject:	Main Building - Snow Load

Thermal Factor	$C_t=1.17$	ASCE 7-22: Table 7.3-3
Ground Snow Load	$p_g=20.00$ psf	ASCE 7-22: Figure s 7.2-1A through 7.2-1D
Balanced Flat Roof Snow Load	$p_f=14.74$ psf	ASCE 7-22: Eq. 7.3-1
Balanced Sloped Roof Snow Load - ASCE 7-22: §7.4		
$p_s = C_s p_f$		ASCE 7-22: Eq. 7.4-1
Roof Obstructions/ Friction	Obstructed	User Defined
Roof Slope	$\theta=1.19^\circ$	User Defined
Roof Snow Factor	$C_s=1.00$	ASCE 7-22: §7.4.1
Balanced Sloped Roof Snow Load	$p_s=14.74$ psf	ASCE 7-22: Eq. 7.4-1
Minimum Snow Load - ASCE 7-22: §7.3.3		
$p_m = \min(p_g, p_{m,max})$ [where $p_{m,max}$ from Table 7.3-4]		ASCE 7-22: Eq. 7.3.3
RC-I: 25 psf		
RC-II: 30 psf		
RC-III: 35 psf		
RC-IV: 40 psf		
Risk Category	II	ASCE 7-22: Table 7.3-4
Ground Snow Load	$p_g=20.00$ psf	ASCE 7-22: Figure s 7.2-1A through 7.2-1D
Minimum Roof Snow Load	$p_m=20.00$ psf	ASCE 7-22: Eq. 7.3.3
Rain on Snow Load - ASCE 7-22: §7.10		
$p_{s,surcharge} = 8$ psf		ASCE 7-22: Eq. 7.10
Balanced Sloped Roof Snow Load	$p_s=14.74$ psf	ASCE 7-22: Eq. 7.4-1
Rain on Snow p_s Surcharge	$p_{s,surcharge}=8.00$ psf	ASCE 7-22: §7.10
Drift Loads for Roof Steps - ASCE 7-22: §7.7		
$\gamma = 0.13p_g + 14 \leq 30$ pcf [Snow density]		ASCE 7-22: Eq. 7.6-1
$h_b = p_s/\gamma$	[Balanced snow load on lower]	
$h_c = h - h_b$	[Clear height from balanced snow to top of obstruction]	
$h_d = 1.5\sqrt{\frac{p_g^{0.74} h_u^{0.70} W_2^{1.7}}{\gamma}}$	[Drift height with W2 parameter]	
$w = 4h_d$	[Snow drift width]	
$p_d = h_d\gamma$	[Max drift load]	
Ground Snow Load	$p_g=20.00$ psf	ASCE 7-22: Figure s 7.2-1A through 7.2-1D
Balanced Flat Roof Snow Load	$p_f=14.74$ psf	ASCE 7-22: Eq. 7.3-1
Balanced Sloped Roof Snow Load	$p_s=14.74$ psf	ASCE 7-22: Eq. 7.4-1
Roof Slope	$\theta=1.19^\circ$	User Defined
Roof Slope Run for A Rise of One	$S=48.14$	Derived from roof slope



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Subject:	Main Building - Snow Load

Foot

Snow Density $\gamma=16.60$ pcf *ASCE 7-22: Figure 7.6-1, 7.7-1, 7.7-2*

Location	Length of roof upwind of the drift (lu)	Length of roof downwind (lu, lower)	Height of roof step (h)	Uniform pressure, windward face (pw)	Uniform pressure, leeward face (pL)	Height of drift (hd)	Pressure surcharge on leeward face (pLS)	Leeward Surcharge width (ds)
E-W East Roof Step	350.0 ft	400.0 ft	6.25 ft	62.72 psf	79.81 psf	3.78 ft	79.81 psf	30.23 ft

* Denoted values are enveloped values between windward and leeward drift cases

Drift Loads for Projections and Parapets - ASCE 7-22: §7.8

$$\gamma = 0.13p_g + 14 \leq 30 \text{ pcf}$$

[Snow density]

ASCE 7-22: Eq. 7.6-1

$$h_b = p_s / \gamma$$

[Balanced snow load on lower]

$$h_c = h - h_b$$

[Clear height from balanced snow to top of obstruction]

$$h_d = 0.75 \times 1.5 \sqrt{\frac{p_g^{0.74} u^{0.70} W_2^{1.7}}{\gamma}}$$

[Windward drift with W2]

$$w = 4h_d$$

[Snow drift width]

$$p_d = h_d \gamma$$

[Max drift load]

Ground Snow Load

 $p_g=20.00$ psf*ASCE 7-22: Figures 7.2-1A through 7.2-1D*

Balanced Flat Roof Snow Load

 $p_f=14.74$ psf*ASCE 7-22: Eq. 7.3-1*

Balanced Sloped Roof Snow Load

 $p_s=14.74$ psf*ASCE 7-22: Eq. 7.4-1*

Roof Slope

 $\theta=1.19^\circ$ *User Defined*

Roof Slope Run for A Rise of One Foot

S=48.14

Derived from roof slope

Snow Density

 $\gamma=16.60$ pcf*ASCE 7-22: §7.8*

Location	Length of roof upwind of the drift (lu)	Length of roof downwind (lu, lower)	Height of roof step (h)	Uniform pressure, windward face (pw)	Uniform pressure, leeward face (pL)	Height of drift (hd)	Pressure surcharge on leeward face (pLS)	Leeward Surcharge width (ds)
E-W West parapet	1.0 ft	400.0 ft	2.25 ft	22.61 psf	7.7 psf	1.36 ft	22.61 psf	10.9 ft
N-S High Parapet	1.0 ft	300.0 ft	5.75 ft	56.72 psf	7.7 psf	3.42 ft	56.72 psf	27.33 ft
N-S Low Parapet	1.0 ft	300.0 ft	2.25 ft	22.61 psf	7.7 psf	1.36 ft	22.61 psf	10.9 ft

* Denoted values are enveloped values between windward and leeward drift cases



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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Seismic Load - Design Criteria

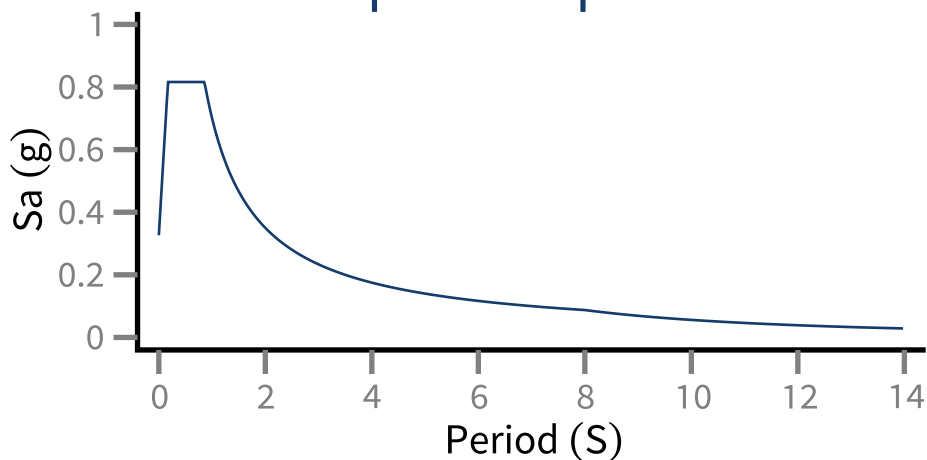
Seismic - Design Criteria

Site Parameters

Building Risk Category	Risk Category = II	ASCE 7-22: §1.5
Seismic Importance Factor	$I_S = 1.0$	ASCE 7-22: Table 1.5-2
Site Class	D	ASCE 7-22: §11.4.2
Special Acceleration at short periods	$S_S = 1.2 \text{ g}$	ASCE 7-22: §11.4.3
Spectral Acceleration at 1s Period	$S_1 = 0.7 \text{ g}$	ASCE 7-22: §11.4.3
Long-Period Transition Period	$T_L = 8.0 \text{ sec}$	ASCE 7-22: §11.4.5.2
MCE _R Spectral Response Acceleration at Short Periods	$S_{MS} = 1.224$	ASCE 7-22: §11.4.3
MCE _R Spectral Response Acceleration at 1s Period	$S_{M1} = 1.05$	ASCE 7-22: §11.4.3
Design Spectral Response Acceleration at Short Periods	$S_{DS} = 0.816$	ASCE 7-22: §11.4.4
Design Spectral Response Acceleration at 1s Period	$S_{D1} = 0.7$	ASCE 7-22: §11.4.4

Horizontal Response Spectrum

Response Spectrum





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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Seismic Load - Building Design

Seismic - Main Force Resisting System

Building Levels / Seismic Weight

Description	Elevation	Seismic Weight at Level, W_x
Roof	22.00 ft	42.25 k
Total Seismic Weight, W		42.2 k

N-S Resisting System

N-S Resisting System System Parameters

System Category	A. BEARING WALL SYSTEMS
System Name	7. ORDINARY PRECAST SHEAR WALLS
Response Modification Coefficient	R=3.0
Overstrength Factor	$\Omega_0=2.5$
Deflection Amplification Factor	$C_d=3.0$

N-S Resisting System Design Summary

Level	Seismic Weight at Level, W_x	Level Force, F_x	Diaphragm Force, F_{px}
Roof	42.25 k	11.49 k	11.49 k

N-S Resisting System Design Details

Approximate Building Period, T_a

$$T_a = C_t h_n^x \quad \text{ASCE 7-22, (12.8-8)}$$

Coefficient for the Fundamental Period	$C_t = 0.02$	ASCE 7-22: Table 12.8-2
Exponent for the Fundamental Period	$x = 0.75$	ASCE 7-22: Table 12.8-2
Height of structure	$h_n = 22.0$	User Defined
Approximate Fundamental Period	$T_a = 0.20$	ASCE 7-22: §12.8.2.1

Base Shear, V

$$V = C_s W \quad \text{ASCE 7-22, (12.8-1)}$$

Seismic Response Coefficient	$C_s = 0.27$	ASCE 7-22: §12.8.1.1
Seismic Weight	$W = 42.25 \text{ k}$	ASCE 7-22: §12.8.1
Seismic Base Shear	$V = 11.49 \text{ k}$	ASCE 7-22: §12.8.1

Vertical Distribution of Seismic Loads to Main Force Resisting System, F_x

$$F_x = C_{vx} V \quad \text{[Lateral seismic force at level]} \quad \text{ASCE 7-22, (12.8-12, 12.8-13)}$$

$$C_{vx} = \frac{w_x h_x^k}{\sum_{i=1}^n w_i h_i^k} \quad \text{[Vertical distribution factor]}$$



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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Seismic Load - Building Design

Level	w_x	h_x	$w_x h_x^k$	C_{vx}	F_x
Roof	42.25 k	22.00 ft	929.50 k	1.00	11.49 k

Vertical Distribution of Seismic Loads to Diaphragm, F_{px}

$$F_{px} = \frac{\sum_{i=x}^n F_i}{\sum_{i=x}^n w_i} w_{px} \quad \text{[Diaphragm design force at level]} \quad \text{ASCE 7-22, (12.10-1, 12.10-2, 12.10-3)}$$

$$F_{px,min} = 0.2 S_{DS} I_e w_{px} \quad \text{[Minimum req'd diaphragm force at level]}$$

$$F_{px,max} = 0.4 S_{DS} I_e w_{px} \quad \text{[Maximum req'd diaphragm force at level]}$$

Level	w_{px}	Σw_i	ΣF_i	$F_{px,min}$	$F_{px,max}$	F_{px}	Design F_{px}
Roof	42.25 k	42.25 k	11.49 k	6.90 k	13.79 k	11.49 k	11.49 k

E-W System

E-W System System Parameters

System Category	H. STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS
System Name	1. STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS
Response Modification Coefficient	R=3.0
Overstrength Factor	$\Omega_0=3.0$
Deflection Amplification Factor	$C_d=3.0$

E-W System Design Summary

Level	Seismic Weight at Level, w_x	Level Force, F_x	Diaphragm Force, F_{px}
Roof	42.25 k	11.49 k	11.49 k

E-W System Design Details

Approximate Building Period, T_a

$$T_a = C_t h_n^x \quad \text{ASCE 7-22, (12.8-8)}$$

Coefficient for the Fundamental Period	$C_t = 0.02$	ASCE 7-22: Table 12.8-2
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Exponent for the Fundamental Period	$x = 0.75$	ASCE 7-22: Table 12.8-2
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Height of structure	$h_n = 22.0$	User Defined
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Approximate Fundamental Period	$T_a = 0.20$	ASCE 7-22: §12.8.2.1
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Base Shear, V

$$V = C_s W \quad \text{ASCE 7-22, (12.8-1)}$$

Seismic Response Coefficient	$C_s = 0.27$	ASCE 7-22: §12.8.1.1
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Subject:	Main Building - Seismic Load - Building Design

Seismic Weight $W = 42.25$ k ASCE 7-22: §12.8.1

Seismic Base Shear $V = 11.49$ k **ASCE 7-22: §12.8.1**

Vertical Distribution of Seismic Loads to Main Force Resisting System, F_x

$$F_x = C_{vx}V \quad \text{[Lateral seismic force at level]} \quad \text{ASCE 7-22, (12.8-12, 12.8-13)}$$

$$C_{vx} = \frac{w_x h_x^k}{\sum_{i=1}^n w_i h_i^k} \quad \text{[Vertical distribution factor]}$$

Level	w_x	h_x	$w_x h_x^k$	C_{vx}	F_x
Roof	42.25 k	22.00 ft	929.50 k	1.00	11.49 k

Vertical Distribution of Seismic Loads to Diaphragm, F_{px}

$$F_{px} = \frac{\sum_{i=x}^n F_i}{\sum_{i=x}^n w_i} w_{px} \quad \text{[Diaphragm design force at level]} \quad \text{ASCE 7-22, (12.10-1, 12.10-2, 12.10-3)}$$

$$F_{px,min} = 0.2S_{DS}I_e w_{px} \quad \text{[Minimum req'd diaphragm force at level]}$$

$$F_{px,max} = 0.4S_{DS}I_e w_{px} \quad \text{[Maximum req'd diaphragm force at level]}$$

Level	w_{px}	Σw_i	ΣF_i	$F_{px,min}$	$F_{px,max}$	F_{px}	Design F_{px}
Roof	42.25 k	42.25 k	11.49 k	6.90 k	13.79 k	11.49 k	11.49 k



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Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - General Requirements

Wind - General Requirements

Site Parameters

Basic Wind Speed	V=115.00 mph	<i>ASCE 7-22: §26.5.1</i>		
Ground Surface Roughness		<i>ASCE 7-22: §26.7.2</i>		
	North Quadrant	South Quadrant	East Quadrant	West Quadrant
	D	D	D	D
Exposure Category		<i>ASCE 7-22: §26.7</i>		
	North Quadrant	South Quadrant	East Quadrant	West Quadrant
	C	C	C	C
Topographic Factors	N/A	<i>ASCE 7-22: §26.8</i>		
Ground Elevation from Sea Level	Z=0.00 ft	<i>User Defined</i>		
Ground Elevation Factor	Ke=1.0000	<i>ASCE 7-22: §26.9</i>		
Building Rigidity Factor	Rigid	<i>ASCE 7-22: §26.11.3, 26.11.4</i>		
Gust Factor	G=0.85	<i>ASCE 7-22: §26.11.1, 26.11.3, 26.11.4</i>		
Enclosure Classification	Enclosed	<i>ASCE 7-22: §26.12</i>		
Internal Pressure Coefficient	+GCpi=0.18, -GCpi=-0.18	<i>ASCE 7-22: Table 26.13-1</i>		



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Subject:	Main Building - Wind Load - MWFRS Directional

Wind – Main Force Resisting System

Region - Main Building

Main Building - Building Information

Main Building - General Configuration

North-South Direction	300.0 ft	User Defined
East-West Direction	400.0 ft	User Defined
Gust Factor	G = 0.85	ASCE 7-22: §26.11.1, 26.11.3, 26.11.4
Internal Pressure Coefficient	GC _{pi} =+/-0.18	ASCE 7-22: §26.13

Main Building - Levels and Parapets

Description	Elevation	Parapet
Level 1	0 ft	No
Roof	22.0 ft	No
S Low Parapet	24.25 ft	Yes
S High Parapet	26.75 ft	Yes

Main Building - Roof Configuration

Roof Shape	Flat	User Defined
Roof slope	1.19°	User Defined
Low point of roof	40.0 ft	User Defined
High point of roof	41.0 ft	User Defined
Mean roof height	40.5 ft	User Defined

Main Building - Wall Design Pressures (MWFRS)

Main Building - Wall Cp Values

Wall Pressure Coefficients

Wind Flow Direction	L/B	C _p , Windward	C _p , Leeward	C _p , Sidewalls
North	0.75	0.8	-0.5	-0.7
South	0.75	0.8	-0.5	-0.7
East	1.33	0.8	-0.43	-0.7
West	1.33	0.8	-0.43	-0.7

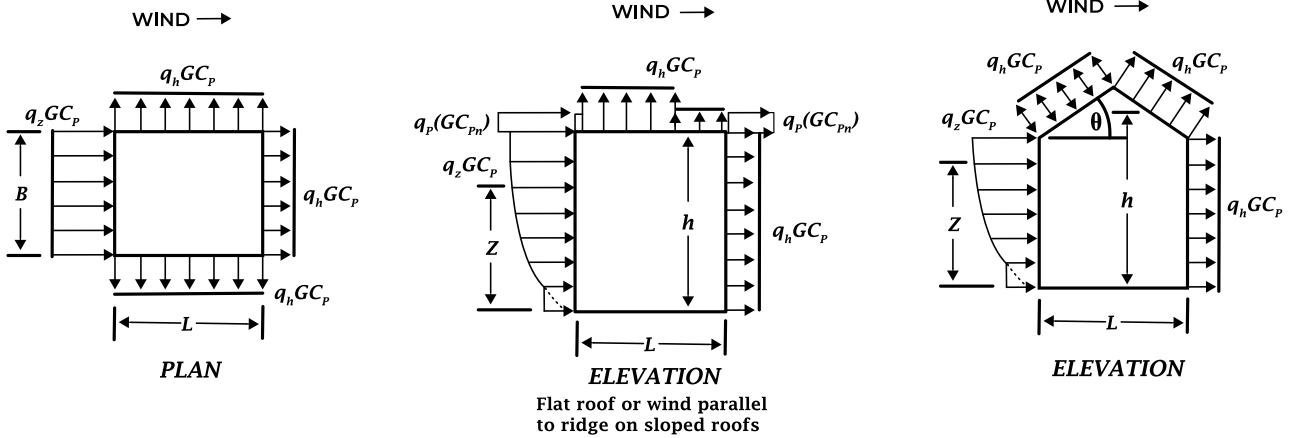
Main Building - Wall Design Pressures – North-South Wind Flow (MWFRS)

$$p = K_d q_h G C_p - K_d q_i (G C_{pi})$$

ASCE 7-22, (27.3-1)



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Case 1 (+GCpi)

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	11.26 psf	-18.15 psf	-23.25 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	12.61 psf	-18.15 psf	-23.25 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	12.97 psf	-18.15 psf	-23.25 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	13.34 psf	-18.15 psf	-23.25 psf

Case 2 (-GCpi)

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	22.06 psf	-7.35 psf	-12.45 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	23.41 psf	-7.35 psf	-12.45 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	23.77 psf	-7.35 psf	-12.45 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	24.14 psf	-7.35 psf	-12.45 psf



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Case Minimum¹

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewall
Level 1	0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
Roof	22.0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S Low Parapet	24.25 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S High Parapet	26.75 ft	N/A	N/A </td <td>N/A</td> <td>16.0 psf</td> <td>0 psf</td> <td>0 psf</td>	N/A	16.0 psf	0 psf	0 psf

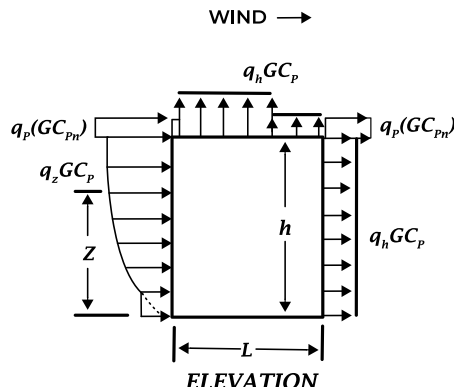
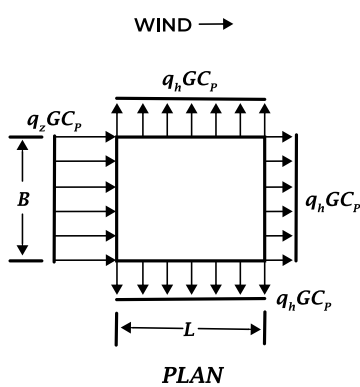
Notes

1. The wind load to be used in the design of the MWFRS for an enclosed or partially enclosed building shall not be less than **16psf** multiplied by the wall area of the building, and **8psf** multiplied by the roof area of the building projected onto a vertical plane normal to the assumed wind direction. Wall and roof loads shall be applied simultaneously. The design wind force for open buildings shall be not less than **16psf** multiplied by the area, A_f .

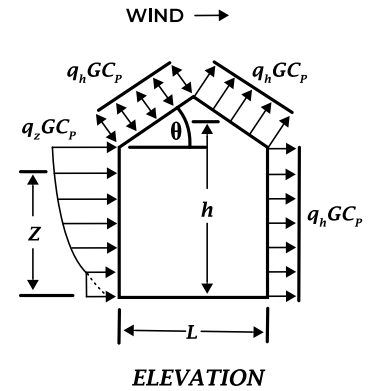
Main Building - Wall Design Pressures - South-North Wind Flow (MWFRS)

$$p = K_d q_h G C_p - K_d q_i (G C_{pi})$$

ASCE 7-22, (27.3-1)



Flat roof or wind parallel to ridge on sloped roofs



FLAT, GABLE, HIP ROOF

Case 1 (+GCpi)



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	11.26 psf	-18.15 psf	-23.25 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	12.61 psf	-18.15 psf	-23.25 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	12.97 psf	-18.15 psf	-23.25 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	13.34 psf	-18.15 psf	-23.25 psf

Case 2 (-GCpi)

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	22.06 psf	-7.35 psf	-12.45 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	23.41 psf	-7.35 psf	-12.45 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	23.77 psf	-7.35 psf	-12.45 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	24.14 psf	-7.35 psf	-12.45 psf

Case Minimum¹

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewall
Level 1	0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
Roof	22.0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S Low Parapet	24.25 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S High Parapet	26.75 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf

Notes

- The wind load to be used in the design of the MWFRS for an enclosed or partially enclosed building shall not be less than **16psf** multiplied by the wall area of the building, and **8psf** multiplied by the roof area of the building projected onto a vertical plane normal to the assumed wind direction. Wall and roof loads shall be applied simultaneously. The design wind force for open buildings shall be not less than **16psf** multiplied by the area, A_f .

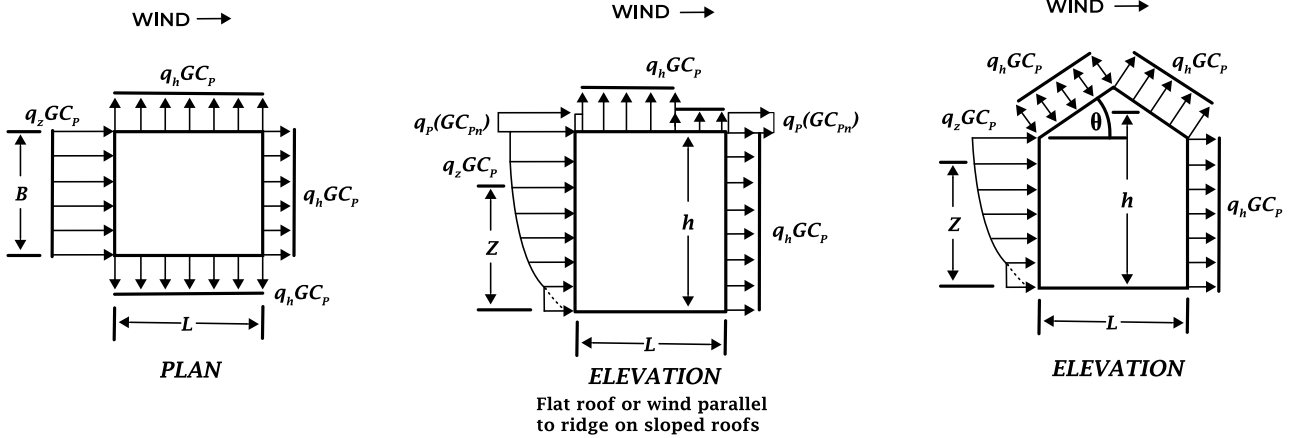
Main Building - Wall Design Pressures - East-West Wind Flow (MWFRS)

$$p = K_d q_h G C_p - K_d q_i (G C_{pi})$$

ASCE 7-22, (27.3-1)



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional



Case 1 (+GCpi)

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	11.26 psf	-18.15 psf	-23.25 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	12.61 psf	-18.15 psf	-23.25 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	12.97 psf	-18.15 psf	-23.25 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	13.34 psf	-18.15 psf	-23.25 psf

Case 2 (-GCpi)

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	22.06 psf	-7.35 psf	-12.45 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	23.41 psf	-7.35 psf	-12.45 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	23.77 psf	-7.35 psf	-12.45 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	24.14 psf	-7.35 psf	-12.45 psf



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional

Case Minimum¹

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewall
Level 1	0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
Roof	22.0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S Low Parapet	24.25 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S High Parapet	26.75 ft	N/A	N/A </td <td>N/A</td> <td>16.0 psf</td> <td>0 psf</td> <td>0 psf</td>	N/A	16.0 psf	0 psf	0 psf

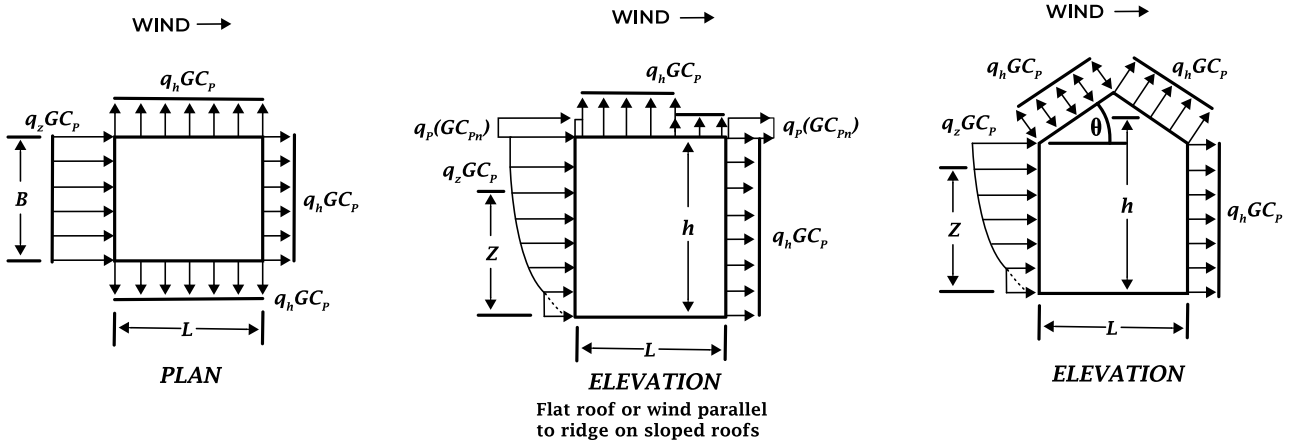
Notes

1. The wind load to be used in the design of the MWFRS for an enclosed or partially enclosed building shall not be less than **16psf** multiplied by the wall area of the building, and **8psf** multiplied by the roof area of the building projected onto a vertical plane normal to the assumed wind direction. Wall and roof loads shall be applied simultaneously. The design wind force for open buildings shall be not less than **16psf** multiplied by the area, A_f .

Main Building - Wall Design Pressures - West-East Wind Flow (MWFRS)

$$p = K_d q_h G C_p - K_d q_i (G C_{pi})$$

ASCE 7-22, (27.3-1)



FLAT, GABLE, HIP ROOF

Case 1 (+GCpi)



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	11.26 psf	-18.15 psf	-23.25 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	12.61 psf	-18.15 psf	-23.25 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	12.97 psf	-18.15 psf	-23.25 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	13.34 psf	-18.15 psf	-23.25 psf

Case 2 (-GCpi)

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewalls
Level 1	0 ft	28.82 psf	35.29 psf	35.29 psf	22.06 psf	-7.35 psf	-12.45 psf
Roof	22.0 ft	31.16 psf	35.29 psf	35.29 psf	23.41 psf	-7.35 psf	-12.45 psf
S Low Parapet	24.25 ft	31.78 psf	35.29 psf	35.29 psf	23.77 psf	-7.35 psf	-12.45 psf
S High Parapet	26.75 ft	32.43 psf	35.29 psf	35.29 psf	24.14 psf	-7.35 psf	-12.45 psf

Case Minimum¹

Level	Elevation	qz	qh	qi	Pwindward	Pleeward	Psidewall
Level 1	0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
Roof	22.0 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S Low Parapet	24.25 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf
S High Parapet	26.75 ft	N/A	N/A	N/A	16.0 psf	0 psf	0 psf

Notes

- The wind load to be used in the design of the MWFRS for an enclosed or partially enclosed building shall not be less than **16psf** multiplied by the wall area of the building, and **8psf** multiplied by the roof area of the building projected onto a vertical plane normal to the assumed wind direction. Wall and roof loads shall be applied simultaneously. The design wind force for open buildings shall be not less than **16psf** multiplied by the area, A_f .

Main Building - Parapet Design Pressures (MWFRS)

Main Building - Parapet Design Pressures (MWFRS) - East - West Wind Flow

$$p_p = K_d q_p G C_{pn}$$

ASCE 7-22, (27.3-5)

Parapet Design Pressures (E/W)

Level	Elevation	Kd	qp	GCpn Windward	GCpn Leeward	Pwindward parapet	Pleeward parapet
S Low Parapet	24.25 ft	0.85	31.78 psf	1.5	-1.0	40.53 psf	-27.02 psf
S High Parapet	26.75 ft	0.85	32.43 psf	1.5	-1.0	41.35 psf	-27.56 psf



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional

Main Building – Parapet Design Pressures (MWFRS) – West - East Wind Flow

$$p_p = K_d q_p G C_{pn}$$

ASCE 7-22, (27.3-5)

Parapet Design Pressures (W/E)

Level	Elevation	Kd	qp	GCpn Windward	GCpn Leeward	Pwindward parapet	Pleeward parapet
S Low Parapet	24.25 ft	0.85	31.78 psf	1.5	-1.0	40.53 psf	-27.02 psf
S High Parapet	26.75 ft	0.85	32.43 psf	1.5	-1.0	41.35 psf	-27.56 psf

Main Building – Parapet Design Pressures (MWFRS) – North - South Wind Flow

$$p_p = K_d q_p G C_{pn}$$

ASCE 7-22, (27.3-5)

Parapet Design Pressures (N/S)

Level	Elevation	Kd	qp	GCpn Windward	GCpn Leeward	Pwindward parapet	Pleeward parapet
S Low Parapet	24.25 ft	0.85	31.78 psf	1.5	-1.0	40.53 psf	-27.02 psf
S High Parapet	26.75 ft	0.85	32.43 psf	1.5	-1.0	41.35 psf	-27.56 psf

Main Building – Parapet Design Pressures (MWFRS) – South - North Wind Flow

$$p_p = K_d q_p G C_{pn}$$

ASCE 7-22, (27.3-5)

Parapet Design Pressures (S/N)

Level	Elevation	Kd	qp	GCpn Windward	GCpn Leeward	Pwindward parapet	Pleeward parapet
S Low Parapet	24.25 ft	0.85	31.78 psf	1.5	-1.0	40.53 psf	-27.02 psf
S High Parapet	26.75 ft	0.85	32.43 psf	1.5	-1.0	41.35 psf	-27.56 psf

Main Building - Roof Design Pressures (MWFRS) ASCE 7-22 27.3.1

Main Building – Roof Cp Values

Case 1 (+GCpi) - Normal and Parallel to Ridge

Wind Flow Direction	h/L	0 to h/2	h/2 to h	h to 2h	> 2h
North	0.14	-0.9	-0.9	-0.5	-0.3
South	0.14	-0.9	-0.9	-0.5	-0.3



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional

East	0.1	-0.9	-0.9	-0.5	-0.3
West	0.1	-0.9	-0.9	-0.5	-0.3

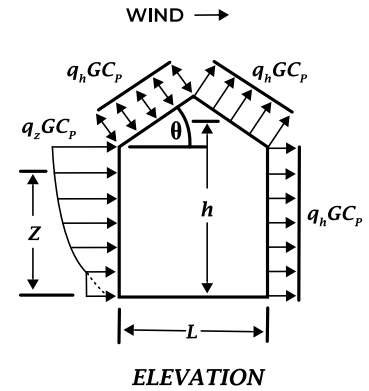
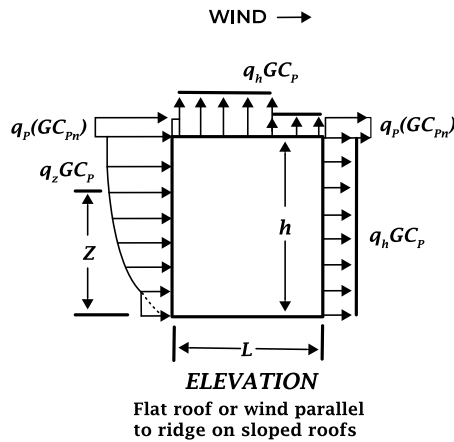
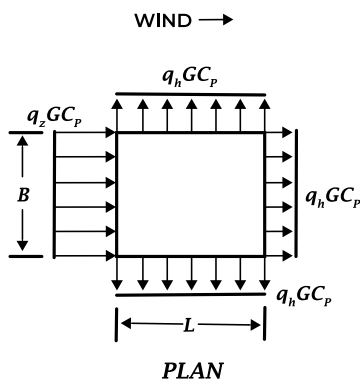
Case 2 (-GCpi) - Normal and Parallel to Ridge

Wind Flow Direction	h/L	0 to h/2	h/2 to h	h to 2h	> 2h
North	0.14	-0.18	-0.18	-0.18	-0.18
South	0.14	-0.18	-0.18	-0.18	-0.18
East	0.1	-0.18	-0.18	-0.18	-0.18
West	0.1	-0.18	-0.18	-0.18	-0.18

Main Building - Roof Design Pressures (MWFRS)

$$p = K_d q_h G C_p - K_d q_i (G C_{pi})$$

ASCE 7-22, (27.3-1)



FLAT, GABLE, HIP ROOF

Case 1 (+GCpi)

Wind Flow Direction	qh & qi	0 to h/2	h/2 to h	h to 2h	> 2h
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Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - MWFRS Directional

North	35.29	-28.35 psf	-28.35 psf	-18.15 psf	-13.05 psf
South	35.29	-28.35 psf	-28.35 psf	-18.15 psf	-13.05 psf
East	35.29	-28.35 psf	-28.35 psf	-18.15 psf	-13.05 psf
West	35.29	-28.35 psf	-28.35 psf	-18.15 psf	-13.05 psf

Case 2 (-GCPI)

Wind Flow Direction	qh & qi	0 to h/2	h/2 to h	h to 2h	> 2h
North	35.29	0.81 psf	0.81 psf	0.81 psf	0.81 psf
South	35.29	0.81 psf	0.81 psf	0.81 psf	0.81 psf
East	35.29	0.81 psf	0.81 psf	0.81 psf	0.81 psf
West	35.29	0.81 psf	0.81 psf	0.81 psf	0.81 psf

Case Minimum^{1,2}

Wind Flow Direction	qh & qi	0 to h/2	h/2 to h	h to 2h	> 2h
North	N/A	8.0 psf	8.0 psf	8.0 psf	8.0 psf
South	N/A	8.0 psf	8.0 psf	8.0 psf	8.0 psf
East	N/A	8.0 psf	8.0 psf	8.0 psf	8.0 psf
West	N/A	8.0 psf	8.0 psf	8.0 psf	8.0 psf

Notes

1. The wind load to be used in the design of the MWFRS for an enclosed or partially enclosed building shall not be less than **16psf** multiplied by the wall area of the building, and **8psf** multiplied by the roof area of the building projected onto a vertical plane normal to the assumed wind direction. Wall and roof loads shall be applied simultaneously. The design wind force for open buildings shall be not less than **16psf** multiplied by the area, A_f .
2. This load shall be applied horizontally onto the projected roof surface when considering the minimum load case.



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - Components & Cladding

Components and Cladding

C&C - Walls - Mean Roof Height ≤ 60 ft - Main Building

Mean Roof Height	$h = 22.0$ ft	ASCE 7-22: §26.2
Roof Slope	$\theta = 1.19^\circ$	User Defined
Least Horizontal Dimension	LHD = 300.0 ft	User Defined
Velocity Pressure at mean roof height	$q_h = 31.16$ psf	ASCE 7-22: §26.10
Internal Pressure Coefficient	$GC_{pi} = \pm 0.18$	ASCE 7-22: Table 26.13-1
Edge Distance	$a = 12.0$	ASCE 7-22: §30.3

C&C - Walls - Mean Roof Height ≤ 60ft - Main Building - GC_p ASCE 7-22: §30.3

Effective Area	All Zones (+ GC_p)	4 (- GC_p)	5 (- GC_p)
10.0 sqft	0.9	-0.99	-1.26
100.0 sqft	0.74	-0.83	-0.94
250.0 sqft	0.68	-0.77	-0.82
550.0 sqft	0.63	-0.72	-0.72

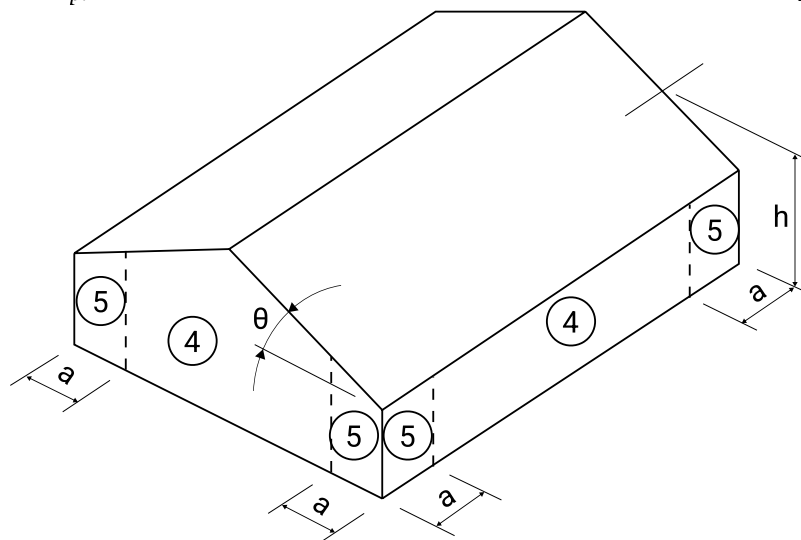
Notes

1. Values of GC_p for walls have been reduced by 10% per Figure 30.3-1 Note 5 ($\theta \leq 10^\circ$).

C&C - Walls - Mean Roof Height ≤ 60ft - Main Building - Design Pressures

$$p = K_d q_h [(GC_p) - (GC_{pi})]$$

ASCE 7-22, (30.3-1)



Case 1 (+ GC_{pi})

Effective Area	All Zones (+ GC_p)	Zone 4 (- GC_p)	Zone 5 (- GC_p)
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Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - Components & Cladding

10.0 sqft	19.07 psf	-30.99 psf	-38.14 psf
100.0 sqft	14.86 psf	-26.78 psf	-29.72 psf
250.0 sqft	13.18 psf	-25.1 psf	-26.37 psf
550.0 sqft	11.92 psf	-23.84 psf	-23.84 psf

Case 2 (-GCpi)

Effective Area	All Zones (+GCp)	Zone 4 (-GCp)	Zone 5 (-GCp)
10.0 sqft	28.6 psf	-21.45 psf	-28.6 psf
100.0 sqft	24.39 psf	-17.24 psf	-20.19 psf
250.0 sqft	22.72 psf	-15.57 psf	-16.83 psf
550.0 sqft	21.45 psf	-14.3 psf	-14.3 psf

Notes

1. Positive numbers mean towards the surface; negative numbers mean away from the surface.
2. Net C&C pressures shall not be less than ± 16 psf

C&C - Roofs

Enclosure Classification	Enclosed	<i>User Defined</i>
Mean Roof Height	h = 22.0 ft	<i>User Defined</i>
Roof Shape	Flat	<i>User Defined</i>
Roof Slope	$\theta = 1.19^\circ$	<i>User Defined</i>
Least Horizontal Dimension	LHD = 300.0 ft	<i>User Defined</i>
Velocity Pressure at mean roof height	qh = 31.16 psf	ASCE 7-22: §26.10
Internal Pressure Coefficient	GCpi = ± 0.18	ASCE 7-22: Table 26.13-1
Edge Distance	a = 12.0	ASCE 7-22: §30.3

C&C - Roof - GCp ASCE 7-22: §30.3

C&C - Roof -- Enclosed Building - GCp

ASCE 7-22: §30.3

Effective Area	All Zones (+GCp)	Zone 1	Zone 1'	Zone 2	Zone 3
10.0 sqft	0.3	-1.7	-0.9	-2.3	-3.2
20.0 sqft	0.27	-1.58	-1.25	-2.14	-2.88
50.0 sqft	0.23	-1.41	-1.05	-1.93	-2.46
100.0 sqft	0.2	-1.29	-0.9	-1.77	-2.14
200.0 sqft	0.2	-1.16	-0.75	-1.61	-1.82

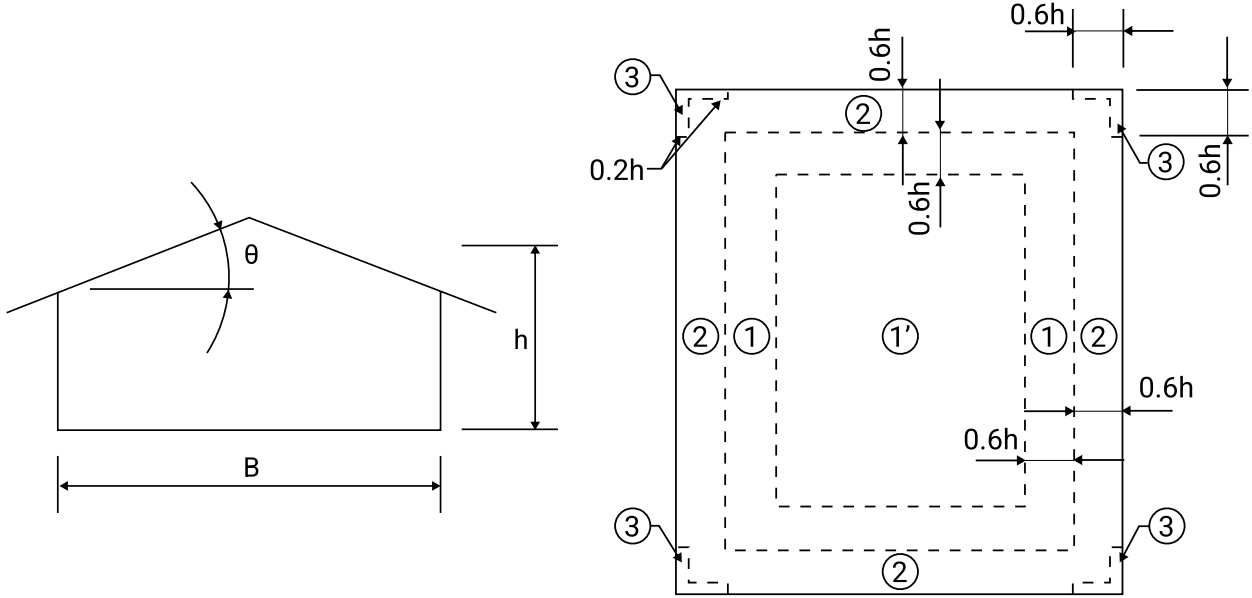
C&C - Roof - Design Pressures



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - Components & Cladding

$$p = K_d q_h [(GC_p) - (GC_{pi})]$$

ASCE 7-22, (30.3-1)



Case 1 (+GC_{pi})

Effective Area	All Zones (+GC _p)	Zone 1	Zone 1'	Zone 2	Zone 3
10.0 sqft	3.18 psf	-49.79 psf	-28.6 psf	-65.68 psf	-89.52 psf
20.0 sqft	2.38 psf	-46.51 psf	-37.86 psf	-61.46 psf	-81.07 psf
50.0 sqft	1.33 psf	-42.17 psf	-32.59 psf	-55.88 psf	-69.91 psf
100.0 sqft	0.53 psf	-38.88 psf	-28.6 psf	-51.65 psf	-61.46 psf
200.0 sqft	0.53 psf	-35.6 psf	-24.62 psf	-47.43 psf	-53.01 psf

Case 2 (-GC_{pi})

Effective Area	All Zones (+GC _p)	Zone 1	Zone 1'	Zone 2	Zone 3
10.0 sqft	12.71 psf	-40.26 psf	-19.07 psf	-56.15 psf	-79.99 psf
20.0 sqft	11.92 psf	-36.97 psf	-28.33 psf	-51.93 psf	-71.54 psf
50.0 sqft	10.86 psf	-32.63 psf	-23.06 psf	-46.34 psf	-60.37 psf
100.0 sqft	10.06 psf	-29.35 psf	-19.07 psf	-42.12 psf	-51.92 psf
200.0 sqft	10.06 psf	-26.06 psf	-15.08 psf	-37.9 psf	-43.48 psf

Notes

1. Positive numbers mean towards the surface; negative numbers mean away from the surface.
2. Net C&C pressures shall not be less than ±16psf

C&C - Parapets - Typical Parapet

Mean Roof Height	h = 22.0 ft	ASCE 7-22: §26.2
Roof Shape	Flat	User Defined



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - Components & Cladding

Roof Slope	$\theta = 1.19^\circ$	User Defined
Structure Width	B = 300.0 ft	User Defined

C&C - Parapet - Typical Parapet - GCp ASCE 7-22: §30.6

Top of Parapet Elevation	Effective Area	Wall Zone below parapet	Roof Zone at parapet ¹	Parapet Porosity	P1	P2	P3	P4
24.25 ft	10.0 sqft	Zone 4	Zone 2	Fully Enclosed	0.9	-2.3	0.9	-0.99
24.25 ft	33.0 sqft	Zone 5	Zone 3	Fully Enclosed	0.82	-2.65	0.82	-1.1
26.75 ft	10.0 sqft	Zone 4	Zone 2	Fully Enclosed	0.9	-2.3	0.9	-0.99
26.75 ft	33.0 sqft	Zone 5	Zone 3	Fully Enclosed	0.82	-2.03	0.82	-1.1

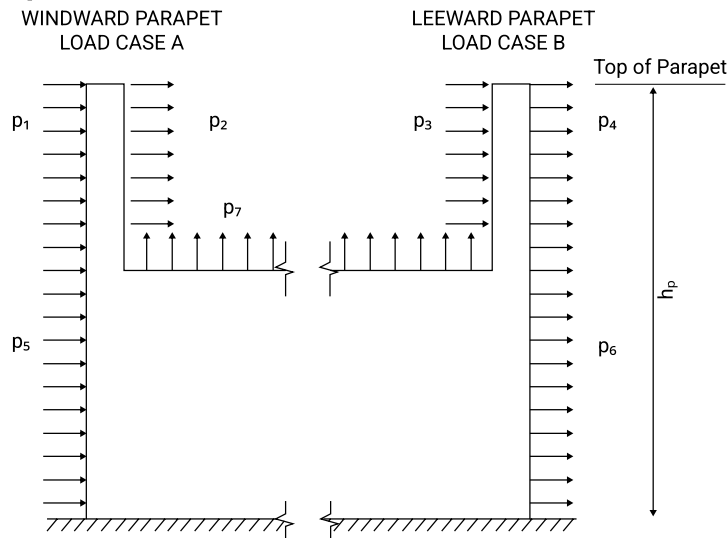
Notes

1. If more than one zone type exists for a particular zone (e.g. zone 2e and 2r), the worst case GCp is used for that zone.
2. GCp values are determined from the applicable wall and roof figures from the row's information provided.

C&C - Parapet - Typical Parapet - Design Pressures

$$p = K_d q_p [(GC_p) - (GC_{pi})]$$

ASCE 7-22, (30.6-1)



Case 1 (+GCpi)

Top of Parapet Elevation	Effective Area	Wall Zone below parapet	Roof Zone at parapet ¹	Parapet Porosity	P1	P2	P3	P4
24.25 ft	10.0 sqft	Zone 4	Zone 2	Fully	19.45 psf	-67.0 psf	19.45 psf	-31.61 psf



Project Title:	Bob's Tile Depot
Engineer:	ACB
Project Id:	26-1234
Structural Design Loads per:	IBC 2024
Subject:	Main Building - Wind Load - Components & Cladding

Enclosed								
24.25 ft	33.0 sqft	Zone 5	Zone 3	Fully Enclosed	17.23 psf	-76.48 psf	17.23 psf	-34.45 psf
26.75 ft	10.0 sqft	Zone 4	Zone 2	Fully Enclosed	19.85 psf	-68.36 psf	19.85 psf	-32.25 psf
26.75 ft	33.0 sqft	Zone 5	Zone 3	Fully Enclosed	17.57 psf	-60.79 psf	17.57 psf	-35.15 psf

Case 2 (-GCpi)

Top of Parapet Elevation	Effective Area	Wall Zone below parapet	Roof Zone at parapet ¹	Parapet Porosity	P1	P2	P3	P4
24.25 ft	10.0 sqft	Zone 4	Zone 2	Fully Enclosed	29.18 psf	-57.28 psf	29.18 psf	-21.88 psf
24.25 ft	33.0 sqft	Zone 5	Zone 3	Fully Enclosed	26.95 psf	-66.75 psf	26.95 psf	-24.73 psf
26.75 ft	10.0 sqft	Zone 4	Zone 2	Fully Enclosed	29.77 psf	-58.43 psf	29.77 psf	-22.33 psf
26.75 ft	33.0 sqft	Zone 5	Zone 3	Fully Enclosed	27.5 psf	-50.86 psf	27.5 psf	-25.23 psf

Notes

1. If more than one zone type exists for a particular zone (e.g. zone 2e and 2r), the worst case GCp is used for that zone.
2. Net C&C pressures shall not be less than +/-16psf